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NATIONAL BUREAU OF STANDARDS REPORT

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QUARTERLY REPORT
ON

EVALUATION OF REFRACTORY QUALITIES OF CONCRETES
FOR JET AIRCRAFT WARM UP, POWER CHECK,
AND MAINTENANCE APRONS

REFERENCE FILE
DO NOT REMOVE

by

W. L. Pendergast, R. A. Clevenger, Edward C. Tuma



U. S. DEPARTMENT OF COMMERCE
NATIONAL BUREAU OF STANDARDS

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NATIONAL BUREAU OF STANDARDS REPORT

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AND MAINTENANCE APRONS

by

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Sponsored by
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U. S. DEPARTMENT OF COMMERCE
NATIONAL BUREAU OF STANDARDS

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FOR JET AIRCRAFT WARM UP, POWER CHECK,
AND MAINTENANCE APRONS

TECHNICAL REQUIREMENTS

The technical requirements are the same as those given in the NBS Report 3012.

1. INTRODUCTION

The objective of the investigation is the determination of the physical properties of concretes that will evaluate their suitability for use in jet aircraft warm-up, power check, and maintenance aprons.

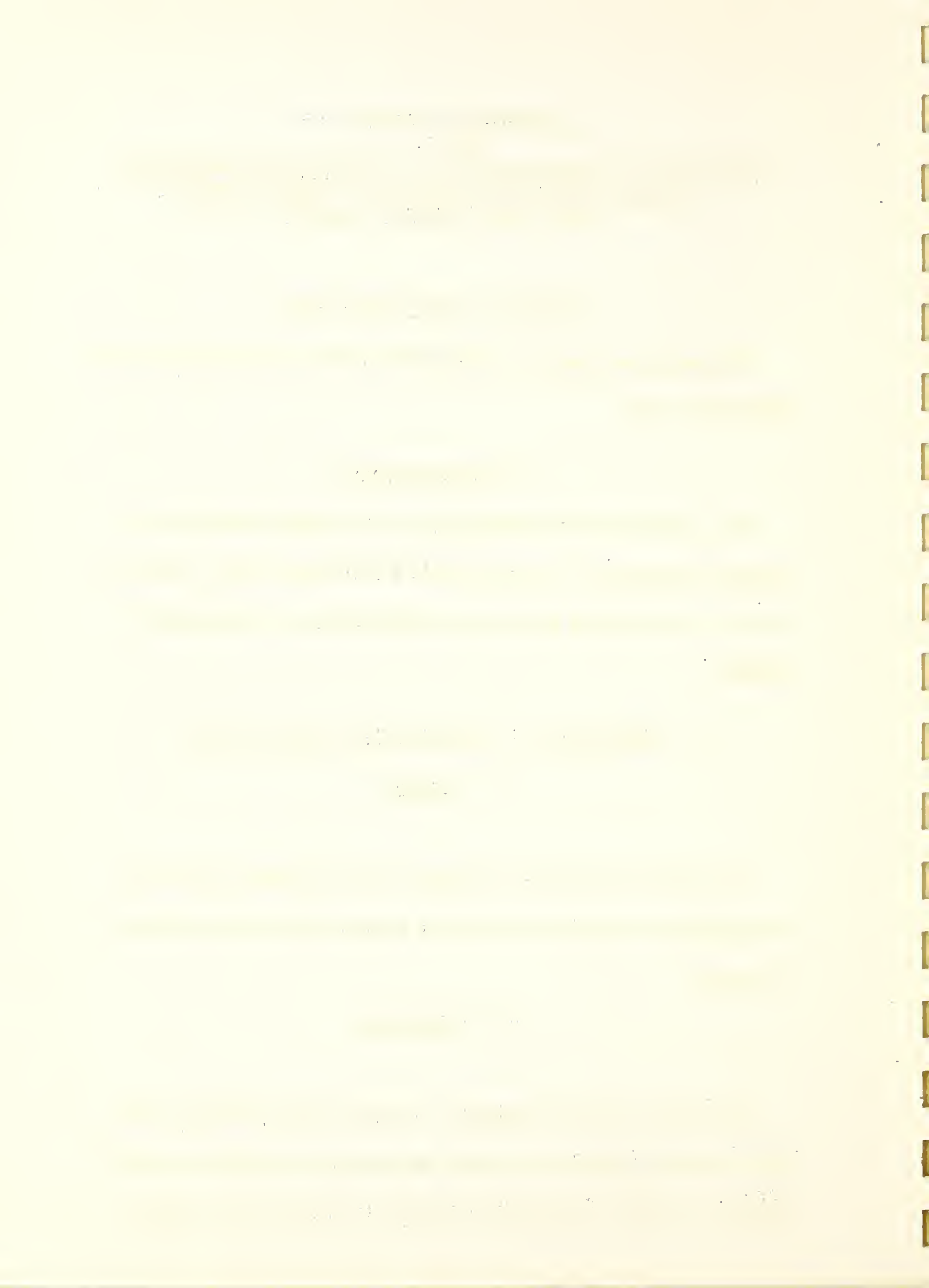
2. MATERIALS: PREPARATION AND TESTING

2.1 Cements

The physical and chemical tests of the cements used in the preparation of concretes during this quarter have been previously reported.

2.2 Aggregates

Six tons of calcined Kentucky flint clay and four tons of raw Kentucky flint clay were crushed and screened during the current quarter. Eleven screens are necessary to grade the aggregate



in accordance with the sizes specified in the Department of the Navy, Bureau of Yards and Docks specification No. 45Ya-1'. Three tons of the calcined clay and two of the raw clay have been screened to the required sizes.

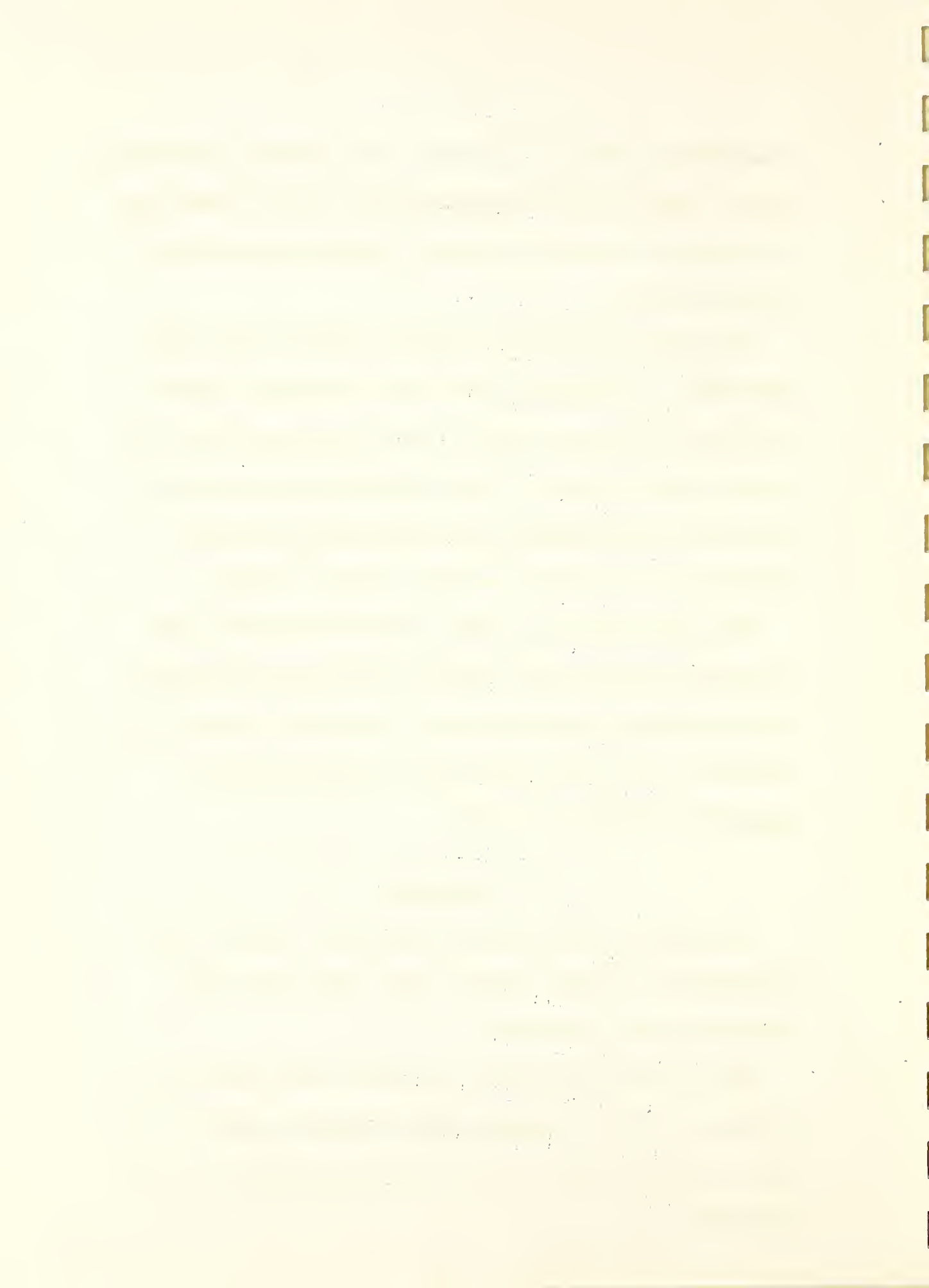
The following properties of both the calcined and flint clay were determined; (1) percent wear in the Los Angeles abrasion test; (2) the bulk specific gravity; (3) the percent absorption; (4) the unit weight in pounds per cubic foot; (5) the correction factor for use with these aggregates when determining the percent entrained air in concrete by use of the pressure method.

Specimens have been cut from pieces of the raw flint clay to be used for flexural and compressive strength determinations. For comparative purposes specimens of the same shape and size were also cut from the building brick, one of the stronger aggregates included in this project.

2.3 Concretes

During the period covered by this report 20 one-cubic foot trial batches of concrete were designed, mixed, specimens fabricated, cured, and tested.

Nine of these trial batches of concretes were designed with the calcined flint clay aggregate, three containing portland cement, three portland pozzolan, and three high-alumina hydraulic cement as the bond.



The remaining 11 of the 20 trial batches of concretes were designed with raw flint clay aggregate. Three of these contained portland cement, three portland pozzolan, and five the high-alumina hydraulic. Using the information obtained from tests on the trial batches three final batches of concrete were designed using calcined clay as the aggregate and one of the three types of cement in each.

The capacity of the mixer, five-cubic feet, necessitated preparing three charges of each concrete in order to yield the 15 cubic-feet used in the fabrication of 5 complete sets of specimens.

One set of specimens of each of the three concretes were cured in the fog-room for 28 days and tested. Four sets of specimens of each concrete were cured in the fog-room for 7 days and stored in the laboratory at ordinary temperatures and humidity and tested. One set of each concrete has been heat-treated at 250°C and 500°C respectively. These six sets have not as yet been tested.

3. RESULTS AND DISCUSSION

3.2 Aggregates

The properties of the six dense aggregates used in designing concretes are given in table 1. The properties of but three of these were determined during the current quarter, the properties of the other three were previously reported and are given again for comparative purposes.

1. The first part of the document discusses the importance of maintaining accurate records of all transactions and activities. It emphasizes that proper record-keeping is essential for transparency and accountability, particularly in the context of financial reporting and auditing.

2. The second part of the document outlines the various methods and techniques used to collect and analyze data. It covers both qualitative and quantitative approaches, highlighting the strengths and limitations of each. The text also discusses the importance of ensuring the reliability and validity of the data collected.

3. The third part of the document focuses on the analysis and interpretation of the data. It discusses various statistical and analytical tools that can be used to identify patterns, trends, and relationships within the data. The text also emphasizes the importance of providing clear and concise interpretations of the results.

4. The fourth part of the document discusses the application of the findings to real-world scenarios. It provides examples of how the data analysis can be used to inform decision-making and to identify areas for improvement. The text also discusses the importance of communicating the results effectively to stakeholders.

5. The final part of the document provides a summary of the key points discussed and offers some concluding thoughts on the importance of data analysis in various fields. It also includes a list of references and a glossary of key terms.

TABLE 1. PROPERTIES OF AGGREGATES

Aggregate	Size	Bulk specific gravity ^{a/b/} S-SD	Water absorption in percent ^{a/} by weight	Los Angeles abrasion percentage wear
White Marsh	gravel	2.64	0.30	40.5
	sand	2.63	0.30	
Bluestone	coarse	2.76	1.50	21.3
	fine	2.65	0.27	
West Virginia hard face brick	coarse	2.46	1.76	27.0
	fine	2.60	0.60	
Olivine	coarse	3.17	0.70	59.7
	fine	3.16	0.20	
Kentucky calcined flint clay	coarse	2.63	1.23	41.3
	fine	2.70	0.70	
Raw Kentucky flint clay	coarse	2.49	5.00	30.0
	fine	2.43	5.88	

^{a/} The gradation of the aggregate was the same as that used in designing the concretes.

^{b/} "S-SD" = saturated surface-dry basis.

The bulk specific gravities (saturated Surface-Dry Basis) of five of the aggregates compared favorably with that of the natural aggregate, gravel and sand, but olivine was slightly heavier. The water absorption was within the normal limits for aggregates with the exception of the raw flint clay. In the abrasion test olivine again was the exception, having the least resistance to wear. If fifty percent wear was taken as the maximum permissible loss, Federal Specification No. SS-A-281,^{2/} olivine would fail to meet that requirement. The low abrasion loss on the raw flint clay was due to the lubricating and cushioning effect of the quickly produced fines. Aggregate of this type often have low wear loss.

If the results of the flexural strength tests on specimens cut from the raw flint clay do not approach the required flexural strength of 650 psi for concretes this would indicate that such an aggregate would be unsuitable.

3.3 Concretes

Table 2 gives the composition and some of the properties of the trial batches of the fresh concretes. The flexural strength of the concretes after curing are also given in this table.

Using as a criteria the data obtained on the trial batches of concretes designed with calcined flint clay and either portland, portland pozzolan, or high-alumina hydraulic cement, three final 15 cubic-foot batches were designed, mixed and complete sets of specimens fabricated. These final batches designed with a 7-sack per cubic yard mix were placable at a 2-inch slump and the air contents were in most instances within the specified range of 4 1/2 percent

TABLE 2. PROPERTIES OF FRESH CONCRETE

Identification	Proportions by weight: cement to coarse and to fine aggregate	Cement Content sacks/yd ³ of concrete	Viscol loss by weight of concrete %	Water Content gals/yd ³ of concrete	Air Content		Slump inches	Weight of Fresh Concrete lbs/ft ³	Water Cement Ratio	Remarks Fresh Concrete	Flexural Strength psi	Remarks Cured Concrete
					Gravimetric %	Air Meter %						
L-C-A	1 : 2.63 : 1.75	7.32	0.010	35.2	1.40	2.38	1.25	149.01	0.43	harsh but placable	800 (8 ^{1/2})	few pull-outs mostly aggregate fracture
L-C-B	do	7.27	0.015	36.3	1.40	2.60	1.00	148.47	0.44	placed well	620 (8)	50% pull-outs 50% aggregate fracture
L-C-C	1 : 2.90 : 1.99	6.56	0.015	35.7	3.20	4.52	2.00	145.42	0.48	good workability	510 (73)	all large aggregate pulled out large & small air voids
P-C-A	1 : 3.03 : 1.42	7.19	0.010	35.4	1.58	2.90	0.50	148.47	0.44	harsh but placable	725 (20)	95% fractured aggregate
P-C-B	do	7.16	0.015	35.7	1.74	3.03	1.00	148.03	0.44	good placability	680	all large aggregate pulled out; few large & numerous small air voids
P-C-C	1 : 2.71 : 1.27	7.45	do	37.4	5.01	6.49	3.00	141.92	0.45	easily placed	660	do
Z-C-A	1 : 3.07 : 1.40	7.23	none	37.1	0.20	0.10	0.50	150.22	0.46	harsh but placable	730 (20)	98% fractured aggregate
Z-C-B	do	7.02	0.010	39.3	1.50	2.68	1.00	146.91	0.49	do	670	very few pull-outs numerous air voids
Z-C-C	1 : 3.32 : 1.51	6.66	do	36.1	1.82	3.08	0.63	147.60	0.48	do	725	do
L-C-1	1 : 2.66 : 1.77	7.11	0.010	36.5	2.10	2.88	1.50	147.12	0.46	easily placed bleeds		
L-C-2	do	7.08	do	36.4	2.43	3.48	1.75	147.95	0.46	do	740	very few pull-outs mostly aggregate fracture numerous air voids
L-C-3	do	7.14	do	36.7	1.94	2.68	1.75	147.94	0.46	do		
P-C-1	1 : 3.02 : 1.42	7.05	0.015	37.1	2.41	4.78	1.75	146.16	0.47	very easily placed		
P-C-2	do	6.83	do	35.5	2.98	6.01	2.50	141.53	0.46	do	705	very few pull-outs mostly aggregate fracture
P-C-3	do	7.12	do	35.4	2.38	4.48	2.00	147.12	0.44	do		

Table continued on next page.
Footnotes appear at end of page.

TABLE 2 (Continued). PROPERTIES OF FRESH CONCRETE ^{a/}

Identification ^b	Proportions by weight: cement to coarse and to fine aggregate	Cement Content sacks/yd ³ of concrete	Winsol Resin by Weight of Concrete %	Water Content gals/yd ³ of concrete	Air Content		Slump inches	Weight of Fresh Concrete lbs./ft ³	Water-Cement Ratio	Remarks Fresh Concrete	Flexural Strength psi	Remarks Cured Concrete
					Gravimetric %	Air Meter %						
Z-C-1	1 : 2.97 : 1.35	7.35	0.010	36.4	1.20	2.76	1.5	148.73	0.44	very harsh but placeable	84.0	very few pull-outs mostly aggregate fracture
Z-C-2	do	7.16	do	38.3	2.48	4.08	2.0	145.76	0.47	easily placed		
Z-C-3	do	7.26	do	37.6	1.63	3.28	1.75	147.42	0.46	do		
L-RQ-A	1 : 2.60 : 1.34	7.24	0.015	30.8	3.85	2.16	1.00	139.30	0.38	too dry to place well	24.0 (14) ^{c/}	honey-combed few pull-outs mostly aggregate fracture
L-RQ-C ¹	1 : 2.34 : 1.13	7.75	0.010	32.3	5.00	3.67	0.00	137.12	0.37	do	25.0 (10)	do
L-RQ-C ²	do	7.58	do	38.4	3.69	2.11	d/	136.25	0.45	too wet	26.0 (10)	large aggregate pulled out, air voids
L-RQ-D	1 : 2.30 : 1.18	7.82	do	42.8	1.40	1.15	d/	142.36	0.48	sloppy but easily placed bleeders	38.0 (7)	do
L-RQ-E	1 : 2.34 : 1.18	7.70	0.015	38.8	2.20	1.62	d/	138.43	0.45	do	28-day fog-room curing not completed	
P-RQ-A	1 : 3.13 : 1.47	6.23	0.015	36.4	2.64	2.66	3.00	137.99	0.52	very easily placed		do
P-RQ-C	1 : 2.66 : 1.24	7.00	0.020	34.7	5.10	4.92	2.50	135.07	0.44	easily placed bleeding		do
P-RQ-D	1 : 2.55 : 1.12	7.67	0.015	37.6	1.55	3.08	2.00	140.18	0.44	very easily placed		do
Z-RQ-A	1 : 3.13 : 1.47	6.32	0.010	37.6	0.98	2.01	1.50	140.18	0.53	very easily placed		do
Z-RQ-C	1 : 2.62 : 1.20	7.15	0.015	39.0	2.97	2.82	1.50	137.12	0.48	do		do
Z-RQ-D	1 : 2.54 : 1.14	7.33	do	37.8	3.97	3.43	2.30	136.25	0.46	do		do

^{a/} For convenience the flexural strength of cured specimens are included if tests are completed.

^{b/} The first letters: L = Lummite, a high-alumina hydraulic cement; P = Portland cement; Z = Portland pozzolan cement. The second letters: C = calcined Kentucky flint clay; R = raw Kentucky flint clay.

The third letters denote trial batches.

The numerals 1, 2, and 3 indicate the number of final batches, of the same design but different charges.

^{c/} The number or numbers in parentheses indicates the number of days the concrete was stored in the fog-room before testing.

^{d/} Too thin for slump test.

± 1.5 percent. The flexural strength, as indicated by the trial batches, was well above the required 650 psi.

The results of tests on the trial batches designed with raw flint clay aggregate indicate that the technical requirements for the properties of the fresh concrete such as slump, air content, and placability may be obtained. However, the results of the few flexural strength tests made on concretes designed with this aggregate and high-alumina hydraulic cement are far below the required strength. Modulus of elasticity determination, on specimens fabricated from trial batches of concrete, using this aggregate and either portland or portland pozzolan cement indicate low strength also. An examination of the test specimens after test shows no pull-outs but complete aggregate fracture.

Table 3 gives the properties of the cured and heat-treated concretes.

All of these concretes with the exception of the one designed with olivine aggregate and high-alumina hydraulic cement developed the required flexural strength of 650 psi after a 28-day fog-room curing. Data collected on trial batches of the concrete containing the olivine aggregate and high-alumina hydraulic cement, indicated that an increase in cement content would result in an accompanying increase in flexural strength to the required 650 psi. The type of mineral structure peculiar to olivine (laminar, with weak bond between strata) probably accounts for the comparatively low flexural strength of the three concretes containing this aggregate. An examination of the beams after testing showed that fracture occurred principally in the aggregate and along the natural cleavage lines.



TABLE 3. PROPERTIES OF CURED AND HEAT-TREATED CONCRETES

Identification	Proportions by Weight of Cement to Coarse and to Fine Aggregate	Treatments Preceding Test ¹	Compressive Strength	Flexural Strength	Abrasion		Young's Modulus of Elasticity Dynamic: Longitudinal	Total ² /Linear Change	Total ² /Weight Change
					Weight of dust	Depth of Wear ³			
			psi	psi	grams	inches	lbs./inch ² x 10 ⁶	%	%
L - O \bar{L}	1 : 3.16 : 2.11	1		490	73.25	0.0142	5.894	no change	+0.27
		2		680	28.65	.0036	6.408	no change	-0.43
		3	4300			6.750		no change	+0.55
		4		370	85.70	.0112	6.822		-3.16
		5		115	200.00	.0382	4.397		-0.03
		6		110	432.00	.0756	3.955		-0.06
		7					1.560		-5.49
		8					.883		+0.04
P - O \bar{L}	1 : 3.50 : 1.50	1		530	19.75	.0045	3.803	+0.01	+0.41
		2		665	18.15	.0051	6.142	-0.01	-0.99
		3	4740			7.195		+0.02	+0.61
		4		675	66.50 \bar{L}	.0155	5.363		-3.84
		5		500	96.80 \bar{L}	.0219	4.873		-4.25
		6		70	616.00 \bar{L}	.1138	3.686		-4.69
		7					1.063		-0.13
		8					3.166		-6.90
Z - O	1 : 3.45 : 1.54	1		580	40.30	.0104	3.861	+0.01	+0.38
		2		730	66.50	.0157	6.006	-0.03	-1.31
		3	5200			6.870		+0.03	+0.61
		4		635	72.00 \bar{L}	.0179	5.211		-0.06
		5		445	83.30 \bar{L}	.0200	4.010		-0.09
		6		150	378.00 \bar{L}	.0691	2.555		-5.15
		7					1.119		-5.70
		8					4.980		-7.30
L - B	1 : 2.11 : 1.53	1		470	33.15	.0098	5.282	+0.01	+0.32
		2		665	43.25	.0115	3.020	-0.01	-1.41
		3	6940			5.590		no change	+0.72
		4		300	70.50 \bar{L}	.0179	3.329		-6.07
		5		290	109.70 \bar{L}	.0271	2.154		-6.68
		6					1.643		-9.90
		7					1.505		-11.55
		8					3.218		-11.55
P - B \bar{L}	1 : 3.12 : 1.47	1		435	21.05	.0075	4.750	+0.01	+0.42
		2		685	16.05	.0036	4.619	-0.01	-2.13
		3	4430			5.280		+0.01	+0.51
		4		580	65.15	.0186	3.788		-5.49
		5		395	93.70 \bar{L}	.0233	3.305		-0.03
		6		125	501.00 \bar{L}	.1159	2.763		-5.76
		7					1.610		-6.54
		8					3.309		-7.73
Z - B	1 : 2.79 : 1.27	1		560	1.00	.0013	3.309	+0.01	+0.29
		2		820	4.30	.0023	3.232	-0.01	-1.90
		3	7790			5.292		+0.02	+0.49
		4		680	26.30	.0094	5.598		-0.03
		5		440	51.00	.0128	4.314		-5.68
		6		155	351.50 \bar{L}	.0725	3.545		-6.27
		7					2.535		-6.97
		8					1.918		-8.51

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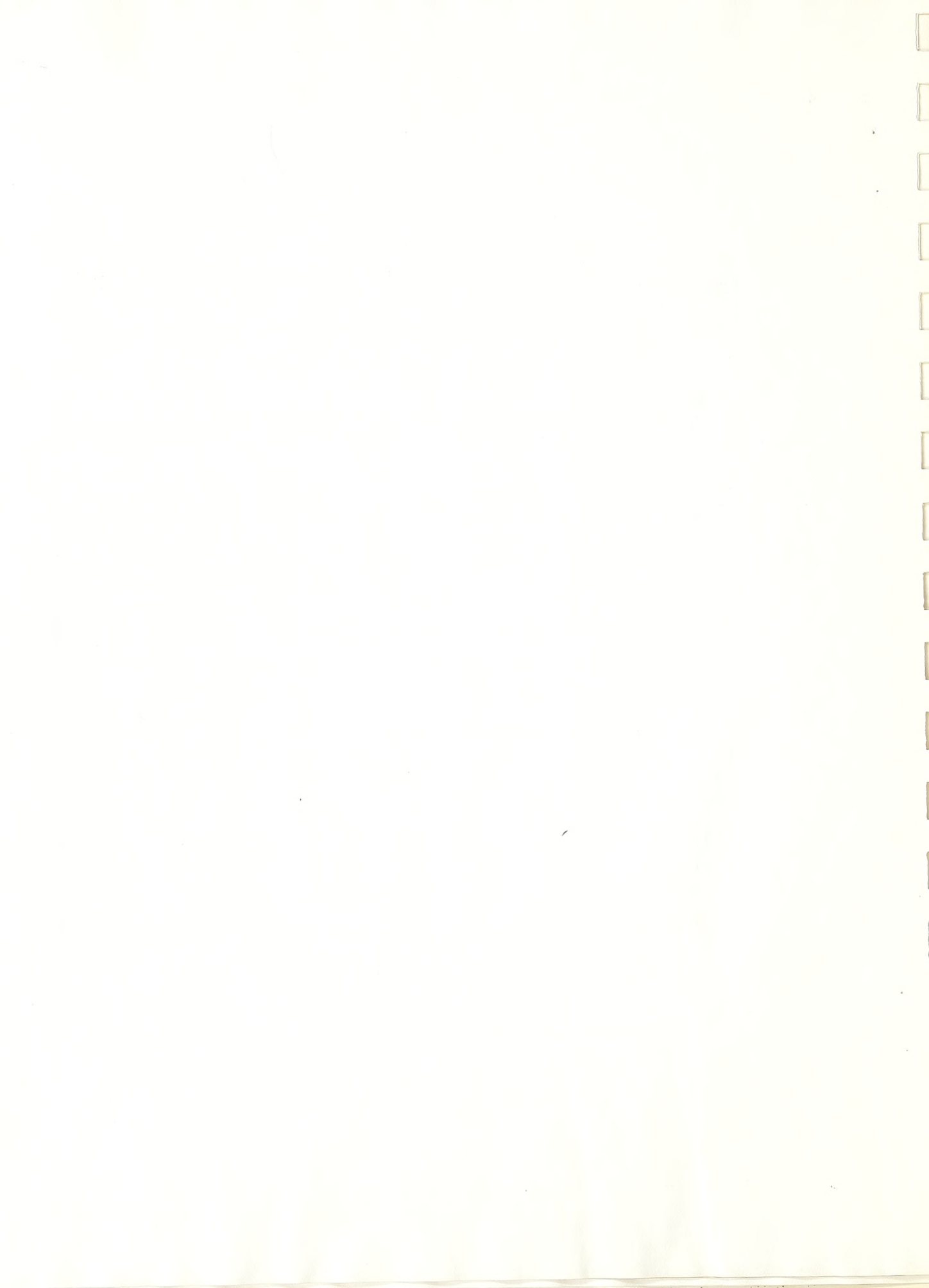


TABLE 3 (Continued). PROPERTIES OF CURED AND HEAT-TREATED CONCRETES

Identification ^{a/}	Proportions by Weight of Cement to Coarse and to Fine Aggregate	Treatments Preceding Test ^{b/}	Compressive Strength	Flexural Strength	Abrasion		Young's Modulus of Elasticity Dynamic: Longitudinal	Total Linear Change	Total Weight Change
					Weight of Dust	Depth of Wear			
			psi	psi	grams	inches	lbs/inch ² x 10 ⁶	%	%
L - C	1 : 2.66 : 1.77	1	5750	565 740	82.10 - 11/	0.0173	6.120 6.352 6.273 6.692 4.429 - -	+0.01 -0.00 +0.02 -0.01 -0.02 -	+0.23 -0.69 +0.66 -3.71 -4.97 -
		2							
		3							
		4							
		5							
		6							
		7							
		8							
P - C	1 : 3.02 : 1.42	1	4420	520 705	34.35	.0094	3.714 5.619 5.309 6.535 - - -	+0.00 -0.02 +0.03 -0.03 -0.05 -	+0.20 -1.65 +0.39 -4.79 -5.33 -
		2							
		3							
		4							
		5							
		6							
		7							
		8							
Z - C	1 : 2.97 : 1.35	1	5570	715 840	25.70	.0063	3.357 5.493 5.129 6.668 - - -	+0.00 -0.01 +0.01 -0.01 -0.04 -	+0.23 -2.17 +0.56 -5.20 -5.68 -
		2							
		3							
		4							
		5							
		6							
		7							
		8							

^{a/} The first letters: L = high-alumina hydraulic cement; P = portland cement; Z = portland pozzolan cement.
The second letters: O = olivine aggregate; B = crushed building brick aggregate; C = calcined Kentucky flint clay.

^{b/} The results in line 1 were obtained after 20 to 24 hours in mold; line 2 after 7 days in fog-room; line 3 after line 2 treatment plus 21 days at ordinary laboratory conditions; line 4 after 28 days in fog-room; line 5 after line 3 treatment plus drying at 110°C; line 6 after line 3 treatment plus heating at 250°C for 5 hours; line 7 after line 3 treatment plus heating at 500°C for 5 hours; line 8 after line 3 treatment plus heating at 1000°C for 5 hours.

^{c/} A description of the apparatus and method used in determining depth of wear was given in NBS Report 3201.

^{d/} Based on length after 24 hours in mold.

^{e/} Based on weight after 24 hours in mold.

^{f/} This data appeared in the last report NBS 3201 and is repeated here for comparative purposes.

^{g/} Coarse aggregate exposed.

^{h/} Dash indicates tests have not been completed.

The results of tests on the concretes designed with crushed building brick aggregate indicate that a 6 1/2 sack mix when portland or portland pozzolan cement is used would develop the required strength.

The three concretes designed with calcined flint clay aggregate and either portland, portland pozzolan, or high-alumina hydraulic cement developed the required flexural strength during the 28-day fog-room curing. These three concretes were designed using a 7-sack mix. An examination of the beams fabricated from this aggregate indicated that failure occurred primarily as aggregate fracture. Fractions of this aggregate screened to the required sizes showed cracking which may have been due to the opening up of natural laminations in the clay during calcining.

The results of tests on the concretes designed with either olivine or crushed building brick as aggregate and either portland or portland pozzolan cement indicate an increase in strength after the 250°C heat exposure and but a slight decrease after the 500°C heat exposure. The linear shrinkage, weight loss, or resistance to abrasion is not effected materially by these heat exposures.

Table 4 gives the results of thermal conductivity measurements * made on specimens of concrete designed with the dense aggregates included in this phase of the project.

* Made by the Heating and Air Conditioning Section, Building Technology Division, National Bureau of Standards.

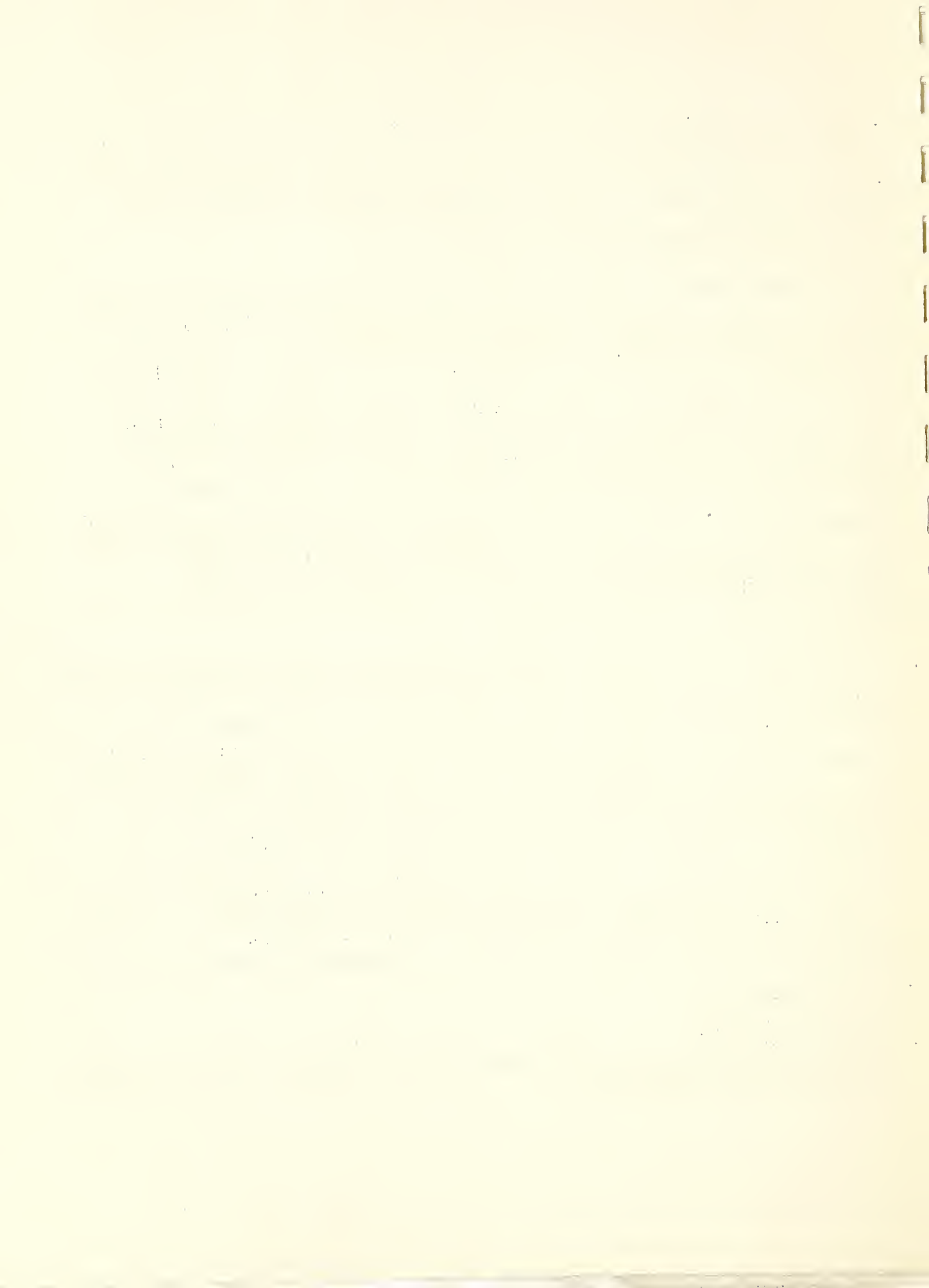


TABLE 4. RESULTS OF THERMAL CONDUCTIVITY MEASUREMENTS

Specimen	Density lb/ft ³	Thermal Conductivity B. T. U./hrft ² (deg F/inch)	Mean Temperature °F
L-WM	134.2	11.3	116
P-WM	131.5	10.4	115
Z-WM	135.0	10.9	116
L-BS	Specimens not suitable for test		
P-BS	139.8	9.1	117
Z-BS	138.0	6.9	117
L-O	146	8.2	116
P-O	149	9.9	117
Z-O	146	8.3	117
L-B	125	5.3	117
P-B	125	6.0	116
Z-B	118	5.3	119

The procedure used in the measurements of thermal conductivity was as follows:

PROCEDURE

Each specimen was dried for at least 8 hours at about 200°F in a ventilated oven immediately prior to the conductivity measurement.

The thermal conductivity of the specimen was measured in an 8-inch pentane hot plate apparatus constructed for making thermal conductivity measurements of concretes. The results of tests in this apparatus have been found to be in close agreement with results obtained in the 8-inch guarded hot plate apparatus conforming with the requirements of Federal Specification LLL-F-321b and of ASTM Test Method C177-45.

The temperature difference from face to face of the specimen during test was measured by means of a thermocouple pressed against each face by a sheet of resilient rubber 0.1 inch thick.

REFERENCES

- 1/ Specification for portland cement concrete pavement for air ports.
Sept. 1952.

- 2/ Aggregate for portland-cement-concrete.

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